

MM2MS3 Mechanics of Solids 3
Exercise Sheet 6 – Strain Energy Methods Solutions

1. Using strain energy, derive an expression for the end deflection of the cantilever beam shown in Fig Q1. $I =$ Second Moment of Area of cross-section and $E =$ Young's Modulus of the beam.

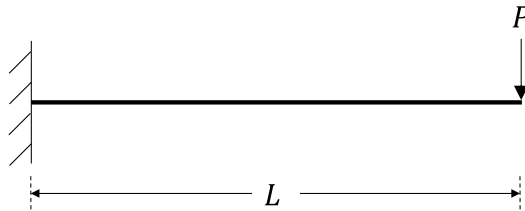
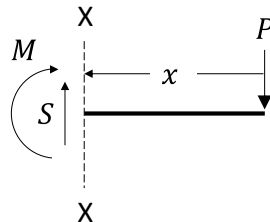


Fig Q1

[Ans: $u_v = \frac{PL^3}{3EI}$]

Solution 1

Free Body Diagram:



Taking moments about X-X:

$$M = Px$$

Substituting this into the equation for Strain Energy in a beam under bending gives,

$$U = \int \frac{M^2}{2EI} ds = \int_0^L \frac{P^2 x^2}{2EI} dx = \frac{P^2}{2EI} \int_0^L x^2 dx = \frac{P^2}{2EI} \left[\frac{x^3}{3} \right]_0^L = \frac{P^2}{2EI} \left(\frac{L^3}{3} - 0 \right)$$
$$\therefore U = \frac{P^2 L^3}{6EI}$$

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End Deflection Calculation for a Cantilever Beam using Castigliano's Theorem

$$u_v = \frac{\delta U}{\delta P}$$

$$\therefore u_v = \frac{PL^3}{3EI}$$

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2. The stepped steel shaft shown in Fig Q2 carries a uniform torque of 500Nm. Determine the total torsional strain energy stored in the shaft. Assume $G_{steel} = 70\text{GPa}$.

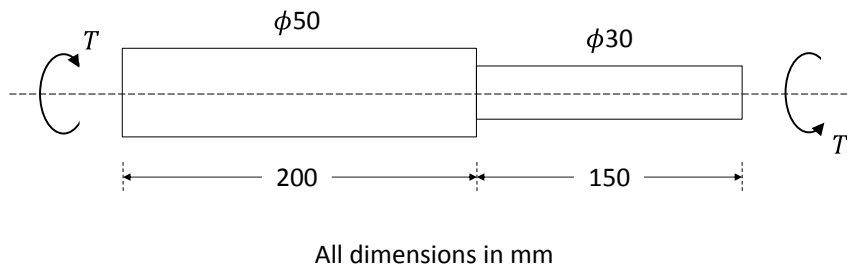


Fig Q2

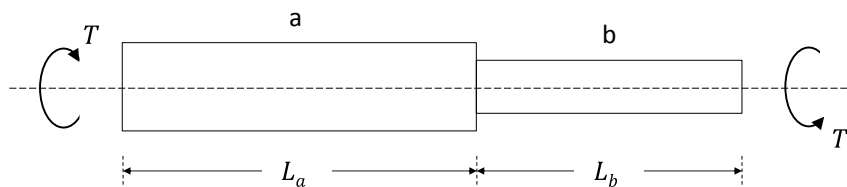
[Ans: 3.95J]

Solution 2

Torsional Strain Energy is given as,

$$U = \int \frac{T^2}{2GJ} ds$$

Labelling each section of the shaft (a and b) and the lengths of the sections, L_a and L_b , respectively, as follows,



Section a

Strain Energy for section a,

$$U_a = \int_0^{L_a} \frac{T^2}{2GJ_a} dx$$

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Section b

Strain Energy for section b,

$$U_b = \int_0^{L_b} \frac{T^2}{2GJ_b} dx$$

Total Strain Energy

$$\begin{aligned} U &= U_a + U_b = \int_0^{L_a} \frac{T^2}{2GJ_a} dx + \int_0^{L_b} \frac{T^2}{2GJ_b} dx \\ &= \frac{T^2}{2GJ_a} \int_0^{L_a} 1 dx + \frac{T^2}{2GJ_b} \int_0^{L_b} 1 dx = \frac{T^2}{2GJ_a} [x]_0^{L_a} + \frac{T^2}{2GJ_b} [x]_0^{L_b} = \frac{T^2}{2GJ_a} (L_a - 0) + \frac{T^2}{2GJ_b} (L_b - 0) \\ &\therefore U = \frac{T^2 L_a}{2GJ_a} + \frac{T^2 L_b}{2GJ_b} \end{aligned} \quad (1)$$

Where,

$$J_a = \frac{\pi d_a^4}{32} = \frac{\pi \times 50^4}{32} = 613,592.32 \text{mm}^4$$

and,

$$J_b = \frac{\pi d_b^4}{32} = \frac{\pi \times 30^4}{32} = 79,521.56 \text{mm}^4$$

Substituting values for T , L_a , L_b , G , J_a and J_b into (1),

$$U = 3950.41 \text{Nmm} = 3.95 \text{Joules}$$

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3. Using strain energy, derive an expression for the deflection at the load point of the beam shown in Fig Q3.

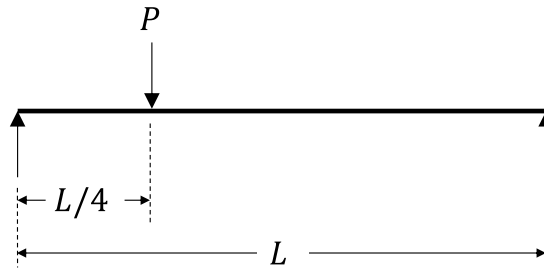
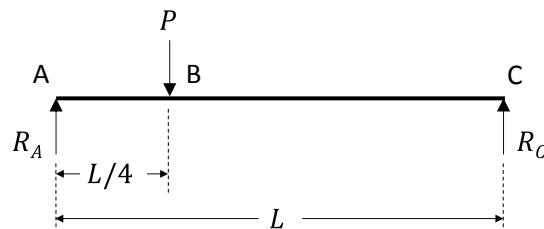


Fig Q3

[Ans: $\frac{9PL^3}{768EI}$]

Solution 3

Find reaction forces as follows:



Vertical equilibrium:

$$R_A + R_C = P \quad (i)$$

Taking moments about A:

$$\frac{PL}{4} = R_C L$$

$$\therefore R_C = \frac{P}{4} \quad (ii)$$

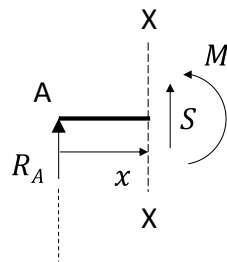
Substituting (ii) into (i) gives:

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$$R_A = \frac{3P}{4} \quad (\text{iii})$$

Section AB (bending only)

Free Body Diagram:



Taking moments about X-X:

$$M_{AB} = R_A x$$

Substituting (iii) into this gives,

$$M_{AB} = \frac{3Px}{4}$$

Substituting this into the equation for Strain Energy in a beam under bending gives,

$$U_{AB} = \int \frac{M_{AB}^2}{2EI} ds = \int_0^{L/4} \frac{\left(\frac{3Px}{4}\right)^2}{2EI} dx = \frac{9P^2}{32EI} \int_0^{L/4} x^2 dx = \frac{9P^2}{32EI} \left[\frac{x^3}{3} \right]_0^{L/4} = \frac{9P^2}{32EI} \left(\frac{L^3}{192} - 0 \right)$$

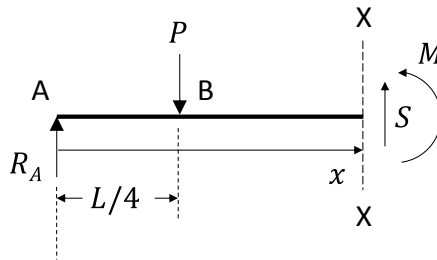
Substituting (iii) into this gives,

$$U_{AB} = \frac{9P^2 L^3}{6144EI}$$

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Section BC (bending only)

Free Body Diagram:



Taking moments about X-X:

$$M_{BC} + P\left(x - \frac{L}{4}\right) = R_A x$$

$$\therefore M_{BC} = R_A x - P\left(x - \frac{L}{4}\right)$$

Substituting (iii) into this gives,

$$M_{BC} = \frac{3Px}{4} - Px + \frac{PL}{4} = \frac{P}{4}(L - x)$$

Substituting this into the equation for Strain Energy in a beam under bending gives,

$$U_{BC} = \int \frac{M_{BC}^2}{2EI} ds = \frac{P^2}{32EI} \int_{L/4}^L (L - x)^2 dx = \frac{P^2}{32EI} \int_{L/4}^L (L^2 - 2Lx + x^2) dx = \frac{P^2}{32EI} \left[L^2 x - Lx^2 + \frac{x^3}{3} \right]_{L/4}^L$$

$$= \frac{P^2}{32EI} \left(L^3 - L^3 + \frac{L^3}{3} - \frac{L^3}{4} + \frac{L^3}{16} - \frac{L^3}{192} \right)$$

$$\therefore U_{BC} = \frac{27P^2 L^3}{6144EI}$$

Total Strain Energy

$$U = U_{AB} + U_{BC} = \frac{9P^2 L^3}{6144EI} + \frac{27P^2 L^3}{6144EI} = \frac{36P^2 L^3}{6144EI}$$

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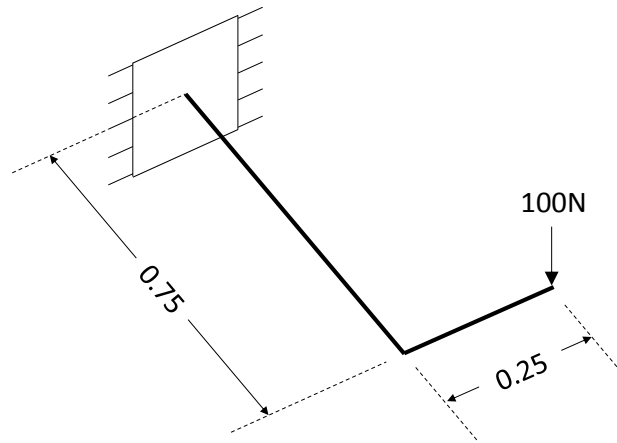
Deflection Calculation using Castigliano's Theorem

$$u_{vB} = \frac{\delta U}{\delta P}$$

$$\therefore u_{vB} = \frac{72PL^3}{6144EI} = \frac{9PL^3}{768EI}$$

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4. Calculate the deflection beneath the force for the cantilevered bracket shown in Fig Q4. The bar is circular in cross section with a diameter, ϕ of 20mm, a Young's Modulus, E of 200GPa and a Shear Modulus, G of 80GPa.



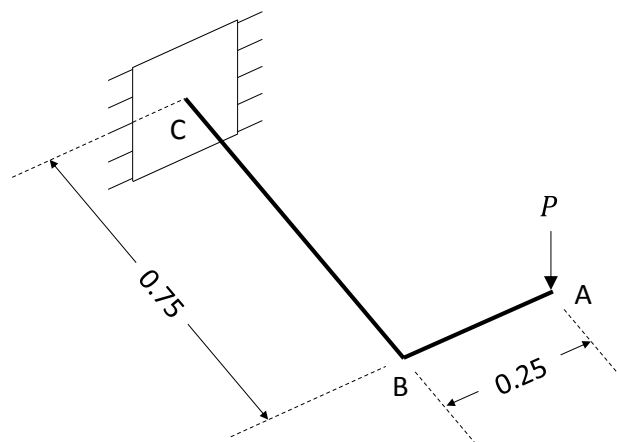
All dimensions in meters

Fig Q4

[Ans: 13mm]

Solution 4

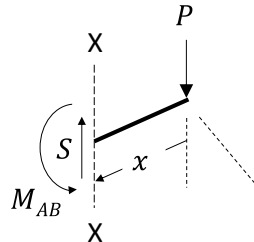
Label lengths and ends of each section:



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Section AB (*bending only*)

Free Body Diagram:



Taking moments about X-X:

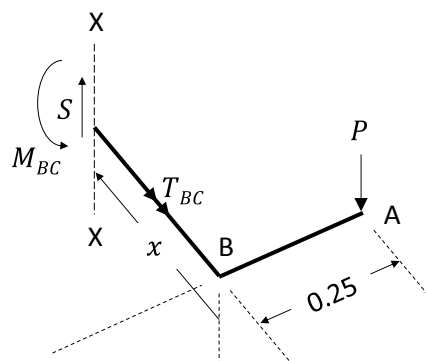
$$M_{AB} = Px$$

Substituting this into the equation for Strain Energy in a beam under bending gives,

$$U_{AB} = \int \frac{M_{AB}^2}{2EI} ds = \int_0^a \frac{(Px)^2}{2EI} dx = \frac{P^2}{2EI} \int_0^a x^2 dx = \frac{P^2}{2EI} \left[\frac{x^3}{3} \right]_0^a = \frac{P^2}{2EI} \left(\frac{a^3}{3} - 0 \right)$$
$$\therefore U_{AB} = \frac{P^2 a^3}{6EI}$$

Section BC

Free Body Diagram:



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Bending

Taking moments about X-X:

$$M_{BC} = Px$$

Substituting this into the equation for Strain Energy in a beam under bending gives,

$$U_{BC}^{bending} = \int \frac{M_{BC}^2}{2EI} ds = \int_0^b \frac{(Px)^2}{2EI} dx = \frac{P^2}{2EI} \int_0^b x^2 dx = \frac{P^2}{2EI} \left[\frac{x^3}{3} \right]_0^b = \frac{P^2}{2EI} \left(\frac{b^3}{3} - 0 \right)$$
$$\therefore U_{BC}^{bending} = \frac{P^2 b^3}{6EI}$$

Torsion

Taking moments about X-X:

$$T_{BC} = Pa$$

Substituting this into the equation for Strain Energy in a beam under bending gives,

$$U_{BC}^{torsion} = \int \frac{T_{BC}^2}{2GJ} ds = \int_0^b \frac{(Pa)^2}{2GJ} dx = \frac{P^2 a^2}{2GJ} \int_0^b 1 dx = \frac{P^2 a^2}{2GJ} [x]_0^b = \frac{P^2 a^2}{2GJ} (b - 0)$$
$$\therefore U_{BC}^{torsion} = \frac{P^2 a^2 b}{2GJ}$$

$$U_{BC} = U_{BC}^{bending} + U_{BC}^{torsion} = \frac{P^2 b^3}{6EI} + \frac{P^2 a^2 b}{2GJ}$$

Total Strain Energy

$$U = U_{AB} + U_{BC} = \frac{P^2 a^3}{6EI} + \frac{P^2 b^3}{6EI} + \frac{P^2 a^2 b}{2GJ}$$
$$\therefore U = \frac{P^2}{2} \left(\frac{a^3}{3EI} + \frac{b^3}{3EI} + \frac{a^2 b}{GJ} \right)$$

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Deflection Calculation using Castigliano's Theorem

$$u_{vA} = \frac{\delta U}{\delta P} = P \left(\frac{a^3}{3EI} + \frac{b^3}{3EI} + \frac{a^2b}{GJ} \right)$$

Where,

$$I = \frac{\pi d^4}{64} = \frac{\pi \times 0.02^4}{64} = 7.85 \times 10^{-9} m^4$$

and,

$$J = \frac{\pi d^4}{32} (= 2I) = 1.57 \times 10^{-8} m^4$$

$$\therefore u_{vA} = 0.013m = 13mm$$

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5. Derive an expression for the increase in distance between the ends A and D of a thin bar of uniform cross-section consisting of a semi-circular portion BC and two straight portions AB and CD as shown in Fig Q5.

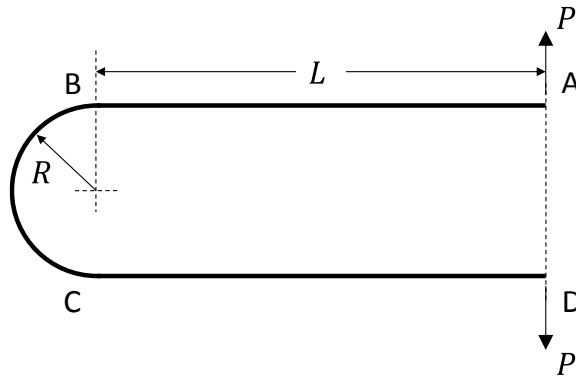


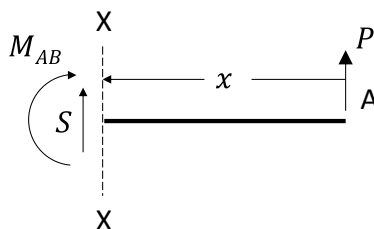
Fig Q5

If the bar is of diameter 6mm, R is 40mm and is to have a spring stiffness, P/δ of 100kg/m, show that the necessary length for L , is approximately 210mm. The bar is made from mild steel with Young's modulus, $E = 210\text{GPa}$.

Solution 5

Section AB (*bending only*)

Free Body Diagram:



Taking moments about X-X:

$$M_{AB} = Px$$

Substituting this into the equation for Strain Energy in a beam under bending gives,

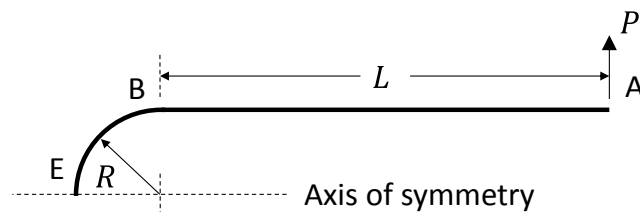
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$$U_{AB} = \int \frac{M_{AB}^2}{2EI} ds = \int_0^L \frac{P^2 x^2}{2EI} dx = \frac{P^2}{2EI} \int_0^L x^2 dx = \frac{P^2}{2EI} \left[\frac{x^3}{3} \right]_0^L = \frac{P^2}{2EI} \left(\frac{L^3}{3} - 0 \right)$$

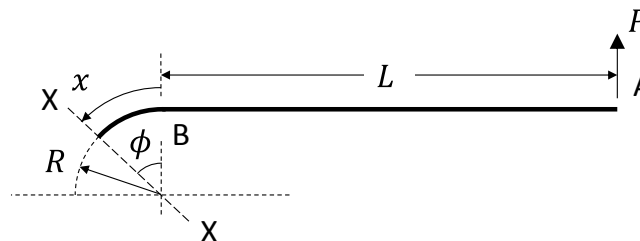
$$\therefore U_{AB} = \frac{P^2 L^3}{6EI}$$

Section BC (bending only)

Due to symmetry, only half of this section must be considered as shown in the diagram below,



For section BE, at angle ϕ , the following Free Body Diagram can be drawn,



Taking moments about X-X:

$$M_{BE} = P(L + R \sin \phi)$$

Substituting this into the equation for Strain Energy in a beam under bending gives,

$$U_{BE} = \int \frac{M_{BE}^2}{2EI} ds = \int_0^{\pi/2} \frac{(P(L + R \sin \phi))^2}{2EI} R d\phi$$

where,

$$dx = R d\phi$$

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Therefore,

$$U_{BE} = \frac{P^2 R}{2EI} \int_0^{\pi/2} (L^2 + 2LR\sin\phi + R^2\sin^2\phi) d\phi \quad (1)$$

Trigonometric Identities:

$$\sin^2\phi + \cos^2\phi = 1 \quad (2)$$

and,

$$\cos 2\phi = \cos^2\phi - \sin^2\phi \quad (3)$$

Rearranging (2) gives,

$$\cos^2\phi = 1 - \sin^2\phi$$

Substituting this into (3) gives,

$$\begin{aligned} \cos 2\phi &= 1 - 2\sin^2\phi \\ \therefore \sin^2\phi &= \frac{1 - \cos 2\phi}{2} \end{aligned}$$

Substituting this into (1) gives,

$$\begin{aligned} U_{BE} &= \frac{P^2 R}{2EI} \int_0^{\pi/2} \left(L^2 + 2LR\sin\phi + \frac{R^2}{2}(1 - \cos 2\phi) \right) d\phi = \frac{P^2 R}{2EI} \left[L^2\phi - 2LR\cos\phi + \frac{R^2}{2}(\phi - \sin\phi\cos\phi) \right]_0^{\pi/2} \\ &= \frac{P^2 R}{2EI} \left(\left(\frac{\pi L^2}{2} - 2LR\cos\left(\frac{\pi}{2}\right) + \frac{R^2}{2} \left(\frac{\pi}{2} - \sin\left(\frac{\pi}{2}\right)\cos\left(\frac{\pi}{2}\right) \right) \right) - (-2LR\cos(0)) \right) \\ \therefore U_{BE} &= \frac{P^2 R}{2EI} \left(\frac{\pi L^2}{2} + \frac{\pi R^2}{4} + 2LR \right) \end{aligned}$$

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Total Strain Energy

The strain energy above the axis of symmetry is:

$$U_{above} = U_{AB} + U_{BE} = \frac{P^2 L^3}{6EI} + \frac{P^2 R}{2EI} \left(\frac{\pi L^2}{2} + \frac{\pi R^2}{4} + 2LR \right) = \frac{P^2}{EI} \left(\frac{L^3}{6} + \frac{\pi L^2 R}{4} + \frac{\pi R^3}{8} + LR^2 \right)$$

As the strain energy below the axis of symmetry is equal to that above, the total strain energy is,

$$U = 2(U_{AB} + U_{BE}) = \frac{P^2}{EI} \left(\frac{L^3}{3} + \frac{\pi L^2 R}{2} + \frac{\pi R^3}{4} + 2LR^2 \right)$$

Vertical Deflection Calculation using Castigliano's Theorem

$$u_{vA} = \delta = \frac{\delta U}{\delta P} = \frac{2P}{EI} \left(\frac{L^3}{3} + \frac{\pi L^2 R}{2} + \frac{\pi R^3}{4} + 2LR^2 \right)$$

Rearranging this for stiffness gives,

$$\frac{P}{\delta} = \frac{EI}{2 \left(\frac{L^3}{3} + \frac{\pi L^2 R}{2} + \frac{\pi R^3}{4} + 2LR^2 \right)} \quad (4)$$

where,

$$I = \frac{\pi d^4}{64} = \frac{\pi \times 6^4}{64} = 63.62 \text{mm}^4$$

Putting $L = 210\text{mm}$ into (4) gives,

$$\therefore \frac{P}{\delta} = 1.02 \frac{\text{N}}{\text{mm}} = 0.104 \frac{\text{kg}}{\text{mm}} \approx 104 \frac{\text{kg}}{\text{m}}$$

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6. Considering the effect of the bending only, determine the horizontal deflection of the point A of the frame shown in Fig Q6 due to the force P.

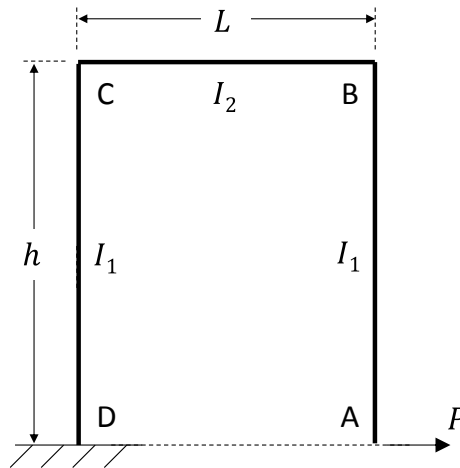


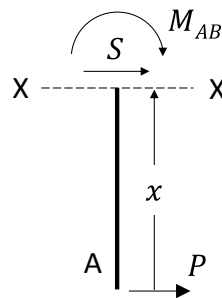
Fig Q6

[Ans: $\frac{Ph^2}{E} \left(\frac{2h}{3I_1} + \frac{L}{I_2} \right)$]

Solution 6

Section AB (*bending only*)

Free Body Diagram:



Taking moments about X-X:

$$M_{AB} = Px$$

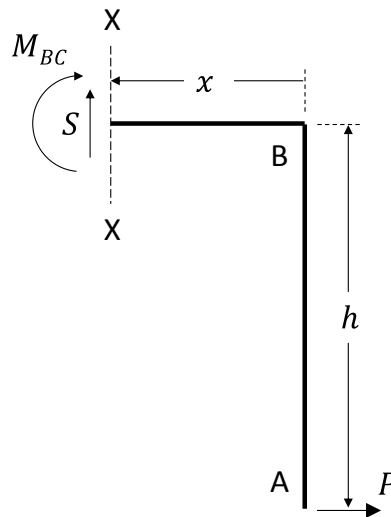
Substituting this into the equation for Strain Energy in a beam under bending gives,

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$$U_{AB} = \int \frac{M_{AB}^2}{2EI_1} ds = \int_0^h \frac{P^2 x^2}{2EI_1} dx = \frac{P^2}{2EI_1} \int_0^h x^2 dx = \frac{P^2}{2EI_1} \left[\frac{x^3}{3} \right]_0^h = \frac{P^2}{2EI_1} \left(\frac{h^3}{3} - 0 \right)$$
$$\therefore U_{AB} = \frac{P^2 h^3}{6EI_1}$$

Section BC (bending only)

Free Body Diagram:



Taking moments about X-X:

$$M_{BC} = Ph$$

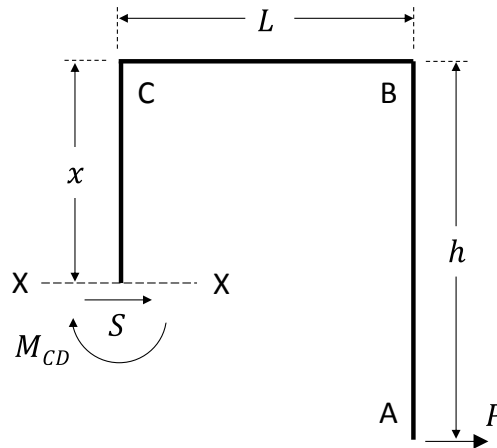
Substituting this into the equation for Strain Energy in a beam under bending gives,

$$U_{BC} = \int \frac{M_{BC}^2}{2EI_2} ds = \int_0^L \frac{P^2 h^2}{2EI_2} dx = \frac{P^2 h^2}{2EI_2} \int_0^L 1 dx = \frac{P^2 h^2}{2EI_2} [x]_0^L = \frac{P^2 h^2}{2EI_2} (L - 0)$$
$$\therefore U_{BC} = \frac{P^2 h^2}{2EI_2} L$$

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Section CD (bending only)

Free Body Diagram:



Taking moments about X-X:

$$M_{CD} = P(h - x)$$

Substituting this into the equation for Strain Energy in a beam under bending gives,

$$\begin{aligned} U_{CD} &= \int \frac{M_{CD}^2}{2EI_1} ds = \int_0^h \frac{(P(h-x))^2}{2EI_1} dx = \int_0^h \frac{P^2(h-x)^2}{2EI_1} dx = \frac{1}{2EI_1} \int_0^h P^2(h^2 - 2hx + x^2) dx \\ &= \frac{P^2}{2EI_1} \left[h^2x - hx^2 + \frac{x^3}{3} \right]_0^h = \frac{P^2}{2EI_1} \left(h^3 - h^3 + \frac{h^3}{3} \right) \\ \therefore U_{CD} &= \frac{P^2 h^3}{6EI_1} \end{aligned}$$

Total Strain Energy

$$U = U_{AB} + U_{BC} + U_{CD} = \frac{P^2 h^3}{6EI_1} + \frac{P^2 h^2}{2EI_2} L + \frac{P^2 h^3}{6EI_1} = \frac{P^2 h^2}{E} \left(\frac{h}{3I_1} + \frac{L}{2I_2} \right)$$

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Deflection Calculation using Castigliano's Theorem

$$u_{vA} = \frac{\delta U}{\delta P}$$

$$\therefore u_{vA} = \frac{Ph^2}{E} \left(\frac{2h}{3I_1} + \frac{L}{I_2} \right)$$

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7. The cranked rod ABCD in Fig Q7 is built-in at end A and carries a transverse force P , perpendicular to the plane ABCD at D. Assuming that the rod is made from round bar of uniform section, obtain (a) the deflection of D in the direction of P and (b) the angular rotation of the end D about axis CD.

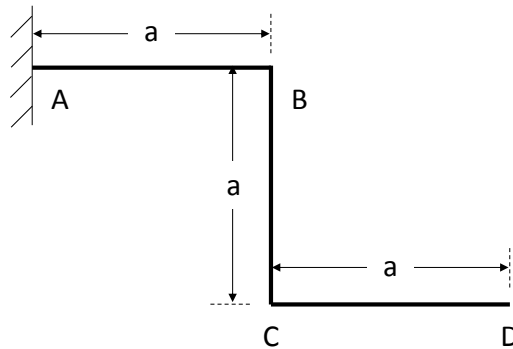
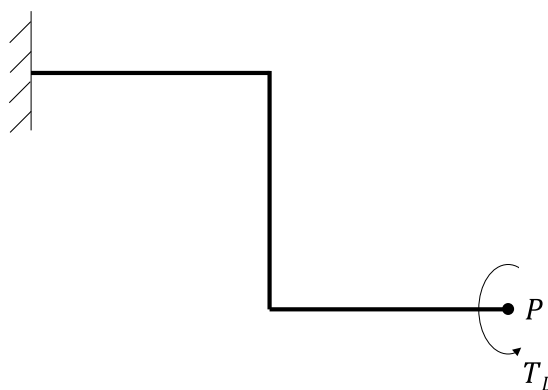


Fig Q7

[Ans: $\frac{Pa^3(3+\frac{E}{G})}{EI}$, $\frac{Pa^2(1+\frac{E}{G})}{2EI}$]

Solution 7

Since the angular displacement is required at position D, a dummy torque must be applied at this position (as well as the applied point load) as follows,



Section CD (*bending and torsion*)

At x from D in CD,

$$M_{CD} = Px$$

and,

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Exercise Sheet 6 – Strain Energy Methods Solutions

$$T_{CD} = T_D$$

$$\begin{aligned} U_{CD} &= U_{CD}^{bending} + U_{CD}^{torsion} = \int \frac{M_{CD}^2}{2EI} ds + \int \frac{T_{CD}^2}{2GJ} ds = \int_0^a \frac{(Px)^2}{2EI} dx + \int_0^a \frac{T_D^2}{2GJ} dx \\ &= \frac{P^2}{2EI} \int_0^a x^2 dx + \frac{T_D^2}{2GJ} \int_0^a 1 dx = \frac{P^2}{2EI} \left[\frac{x^3}{3} \right]_0^a + \frac{T_D^2}{2GJ} [x]_0^a = \frac{P^2}{2EI} \left(\frac{a^3}{3} - 0 \right) + \frac{T_D^2}{2GJ} (a - 0) \\ &\therefore U_{CD} = \frac{P^2 a^3}{6EI} + \frac{T_D^2 a}{2GJ} \end{aligned}$$

Section BC (bending and torsion)

At x from C in BC,

$$M_{BC} = Px + T_D$$

and,

$$T_{BC} = Pa$$

$$\begin{aligned} U_{BC} &= U_{BC}^{bending} + U_{BC}^{torsion} = \int \frac{M_{BC}^2}{2EI} ds + \int \frac{T_{BC}^2}{2GJ} ds = \int_0^a \frac{(Px + T_D)^2}{2EI} dx + \int_0^a \frac{(Pa)^2}{2GJ} dx \\ &= \frac{1}{2EI} \int_0^a (Px + T_D)^2 dx + \frac{P^2 a^2}{2GJ} \int_0^a 1 dx = \frac{1}{2EI} \int_0^a (P^2 x^2 + 2PxT_D + T_D^2) dx + \frac{P^2 a^2}{2GJ} \int_0^a 1 dx \\ &= \frac{1}{2EI} \left[\frac{P^2 x^3}{3} + Px^2 T_D + T_D^2 x \right]_0^a + \frac{P^2 a^2}{2GJ} [x]_0^a \\ &\therefore U_{BC} = \frac{1}{2EI} \left(\frac{P^2 a^3}{3} + Pa^2 T_D + T_D^2 a \right) + \frac{P^2 a^3}{2GJ} \end{aligned}$$

Section AB (bending and torsion)

At x from b in AB,

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$$M_{AB} = P(x + a)$$

and,

$$T_{AB} = T_D + Pa$$

$$\begin{aligned} U_{AB} &= U_{AB}^{bending} + U_{AB}^{torsion} = \int \frac{M_{AB}^2}{2EI} ds + \int \frac{T_{AB}^2}{2GJ} ds \\ &= \int_0^a \frac{(P(x+a))^2}{2EI} dx + \int_0^a \frac{(T_D + Pa)^2}{2GJ} dx = \frac{P^2}{2EI} \int_0^a (x+a)^2 dx + \frac{(T_D + Pa)^2}{2GJ} \int_0^a 1 dx \\ &= \frac{P^2}{2EI} \int_0^a (x^2 + 2ax + a^2) dx + \frac{T_D^2 + 2T_D Pa + P^2 a^2}{2GJ} \int_0^a 1 dx \\ &= \frac{P^2}{2EI} \left[\frac{x^3}{3} + ax^2 + a^2 x \right]_0^a + \frac{T_D^2 + 2T_D Pa + P^2 a^2}{2GJ} [x]_0^a \\ \therefore U_{AB} &= \frac{7P^2 a^3}{6EI} + \frac{T_D^2 a + 2T_D Pa^2 + P^2 a^3}{2GJ} \end{aligned}$$

Total Strain Energy

$$\begin{aligned} U &= U_{AB} + U_{BC} + U_{CD} \\ &= \frac{P^2 a^3}{6EI} + \frac{T_D^2 a}{2GJ} + \frac{1}{2EI} \left(\frac{P^2 a^3}{3} + Pa^2 T_D + T_D^2 a \right) + \frac{P^2 a^3}{2GJ} + \frac{7P^2 a^3}{6EI} + \frac{T_D^2 a + 2T_D Pa^2 + P^2 a^3}{2GJ} \end{aligned}$$

(a) Deflection at D Calculation using Castigliano's Theorem

$$u_{vD} = \frac{\delta U}{\delta P} = \frac{Pa^3}{3EI} + \frac{1}{2EI} \left(\frac{2Pa^3}{3} + a^2 T_D + T_D^2 a \right) + \frac{Pa^3}{GJ} + \frac{7Pa^3}{3EI} + \frac{T_D a^2 + Pa^3}{GJ}$$

Setting dummy torque, T_D , to zero gives,

$$u_{vD} = \frac{3Pa^3}{EI} + \frac{2Pa^3}{GJ}$$

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(b) Angular Rotation at D Calculation using Castigliano's Theorem

$$\theta_D = \frac{\delta U}{\delta T_D} = \frac{T_D^2 a}{2GJ} + \frac{Pa^2 + 2T_D a}{2EI} + \frac{T_D a + Pa^2}{GJ}$$

Setting dummy torque, T_D , to zero gives,

$$\theta_D = \frac{Pa^2}{2EI} + \frac{Pa^2}{GJ}$$

For a circular cross section,

$$I = \frac{\pi d^4}{64} = \frac{\pi \times 0.02^4}{64} = 7.85 \times 10^{-9} m^4$$

and,

$$J = \frac{\pi d^4}{32} (= 2I) = 1.57 \times 10^{-8} m^4$$

$$\therefore J = 2I$$

Therefore,

$$u_{vD} = \frac{Pa^3}{EI} \left(3 + \frac{E}{G} \right)$$

and,

$$\theta_D = \frac{Pa^2}{2EI} \left(1 + \frac{E}{G} \right)$$